



**North American  
Pultrusion Conference**

# **Determination of real life aging on construction products made of glass fibre reinforced plastics (GFRP)**

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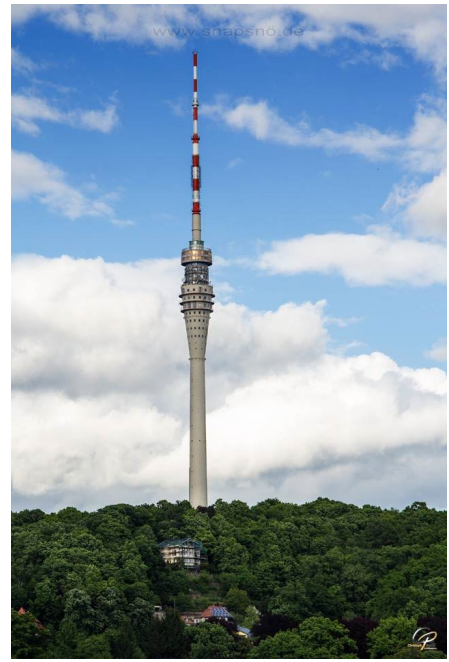
# Content



1. **History: GFRP Antenna Carrier for a Radio Tower**
2. **Factors on Aging of GFRP**
3. **Case Study 1: GFRP-Bridge**
4. **Case Study 2: GFRP-Structured Cooling Tower**
5. **Case Study 3: GFRP-Pipe at Power Plant**
6. **Approach for Failure Determination during Aging Test and Aging**

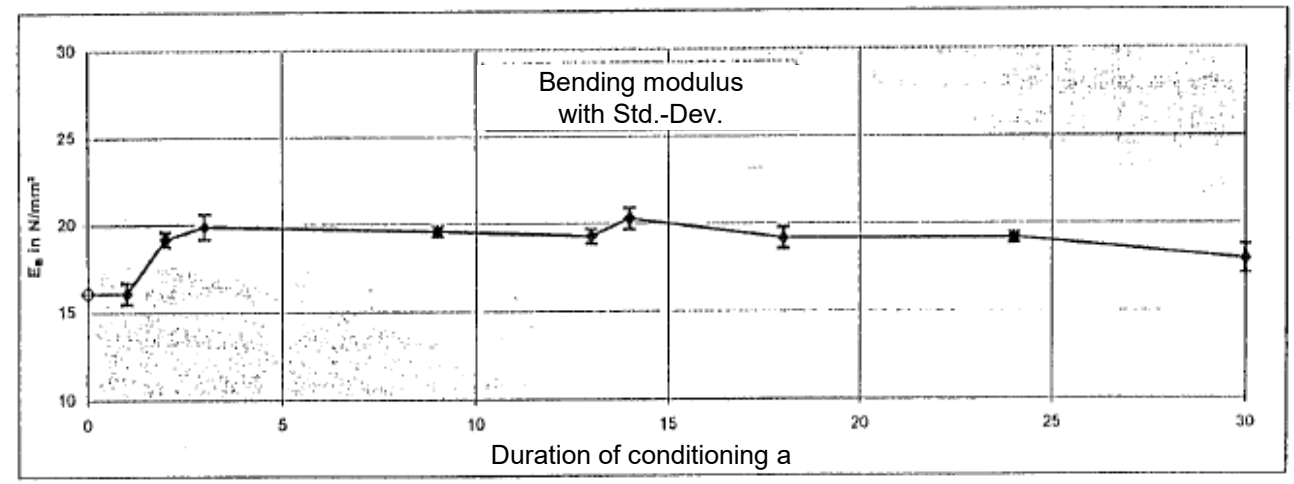
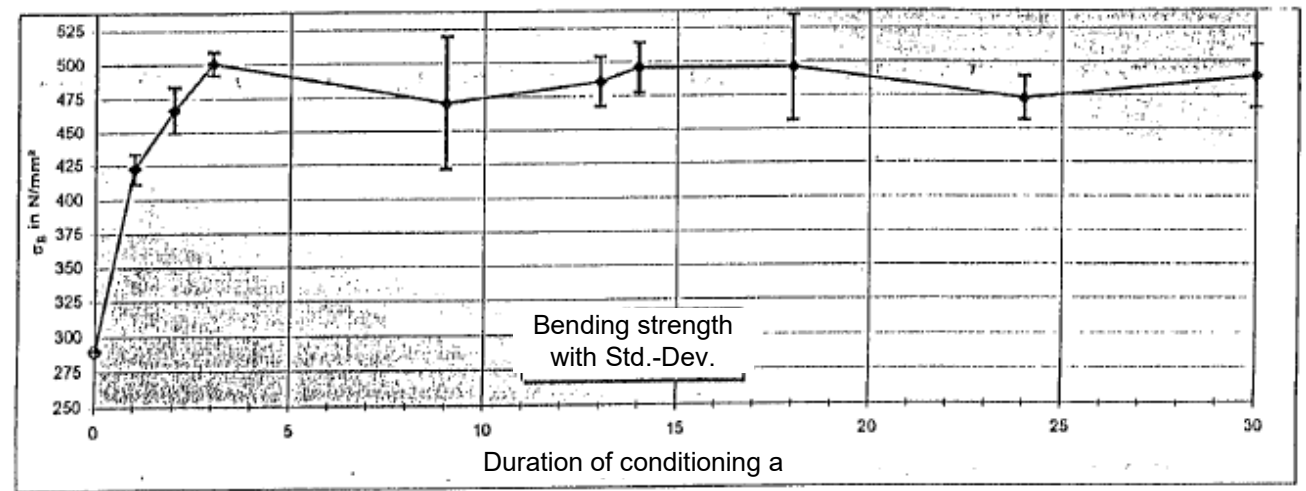
# History: GFRP Antenna Carrier for a Radio Tower

## Monitoring of the GFRP structure by:



- Samples exposed at antenna ("witnesses") over a period of 24 years
- Final testing at samples cut out of GFRP antenna 30 years after installation

Television tower of Dresden, built 1969



Source: IMA Dresden, AVK Conference 1999

# Factors on Aging of GFRP

## Long-term behaviour

- For the construction industry GFRP-components are designed for service lives of up to 100 years.
  - During service live: changing material properties must be taken into account
    - Suitable approach from standard DIN EN 13121-3: **Factors of influence:**

- $A_1$ : **Manufacturing** on the laminate (for advanced design:  $A_1 = 1.0$ )
- $A_2$ : **Medium or environment** on the laminate
- $A_3$ : Design **temperature** on the laminate
- $A_4$ : **Dynamic** (cyclic) **load** on the laminate
- $A_5$ : **Load duration** on the laminate
- $A_6$ : Ionising **radiation**

- $E_d$  ... Design value of the stresses
- $R_d$  ... Load-bearing resistance
- $E_i$  ... Load effect
- $R_k$  ... Laminate resistance
- $\gamma_F$  ... Portion factor of variable impacts
- $\gamma_M$  ... Portion factor for material (= 1.4)

→ Goal for design:  $E_d \leq R_d$

$$E_d = A_5 \cdot E_i \cdot \gamma_{F,i} \leq R_d = \frac{R_k}{\gamma_M \cdot A_1 \cdot A_2 \cdot A_3 \cdot A_4 \cdot A_6}$$



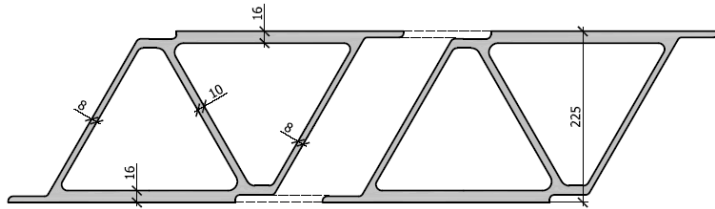
GFRP profiles in cooling tower



# Case Study 1: GFRP-Bridge



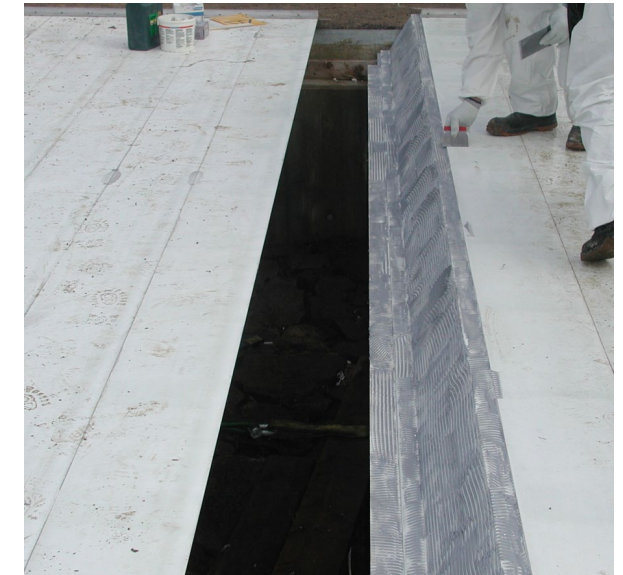
- 1<sup>st</sup> road bridge with GFRP structure in Germany
- Built in 2004 with asset profiles of manufacturer Fiberline Composites A/S
- Pre-assembly at manufacturer, final installation with on-site



GFRP-bridge: During installation; Source: Fiberline A/S



GFRP-bridge after installation;  
Source: Fiberline A/S



Adhesive bonding of GFRP-bridge;  
Source: Fiberline A/S

# Case Study 1: GFRP-Bridge

## IMA Services:

- Materials and Component testing
- Manufacturer inspection with Inspection of pre-assembly
- Inspection of insertion and adhesion
- Measurement of the bridge deformation at load of 20 t
- “Witness” samples for long-term monitoring



GFRP-bridge: After installation with Truck (weight approx. 20 t); Source: IMA



Witness samples inside the bridge; Source: IMA

Test plan of witness samples:

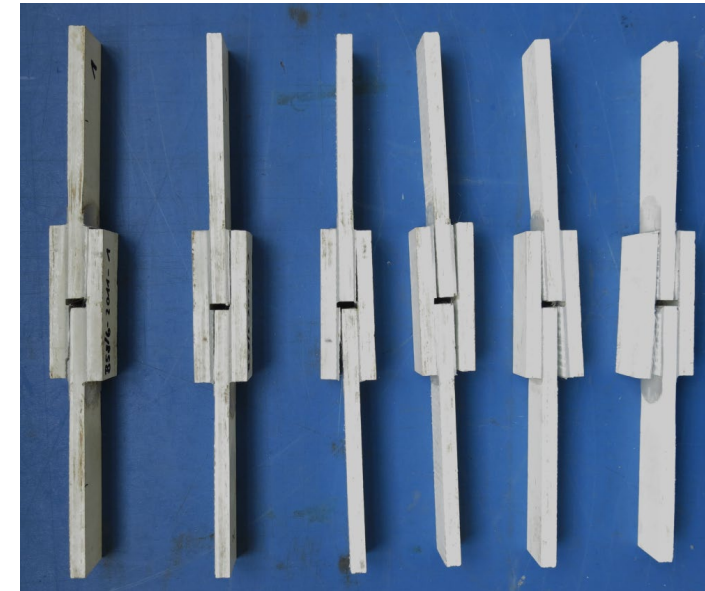
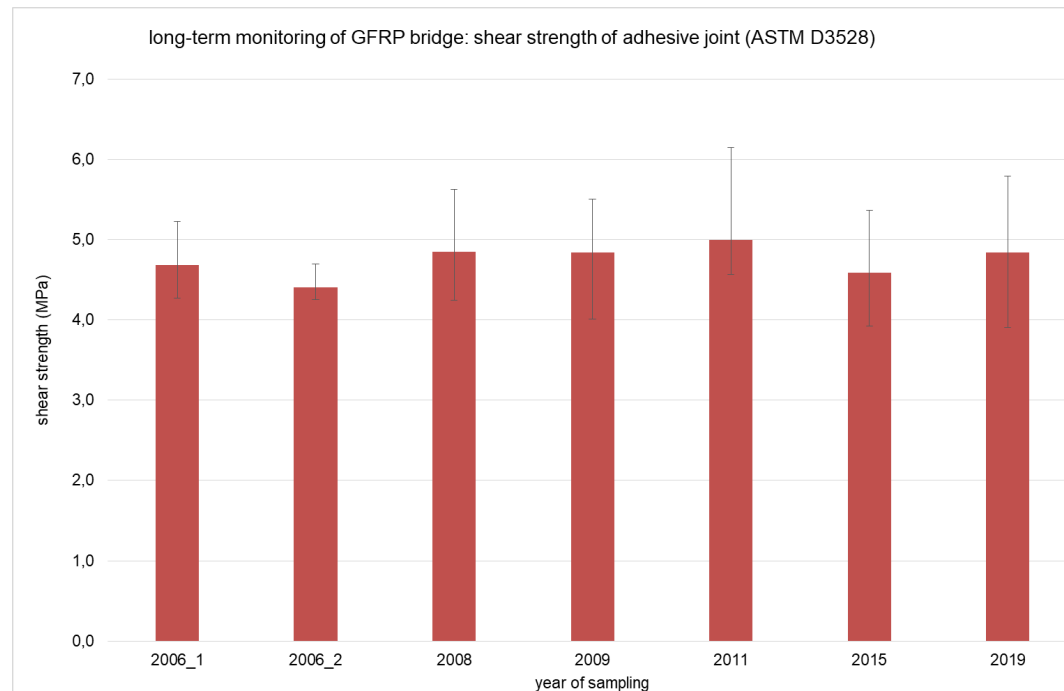
Test / year	2006	2008	2009	2011	2015	2019	2023	2028	2033	2038
<b>DLS</b>	X	X	X	X	X	X	X	X	X	X
<b>Bending</b>	X		X	X	X	X	X	X	X	X
<b>Creep</b>				X		X		X		X



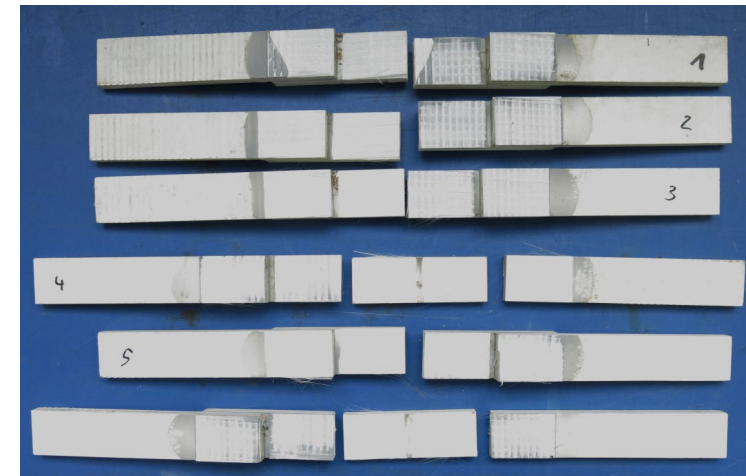
# Case Study 1: GFRP-Bridge

## Bonding properties for practice-aged samples:

- Double lap shear test (DLS) following ASTM D 3528
- Test until shear failure of adhesive



DLS-samples before the test

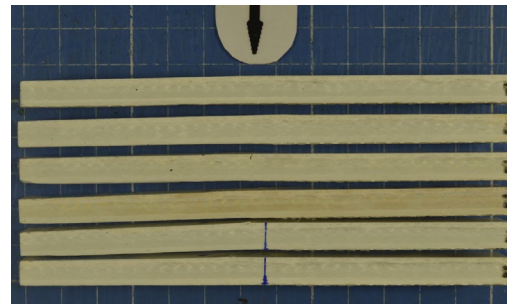
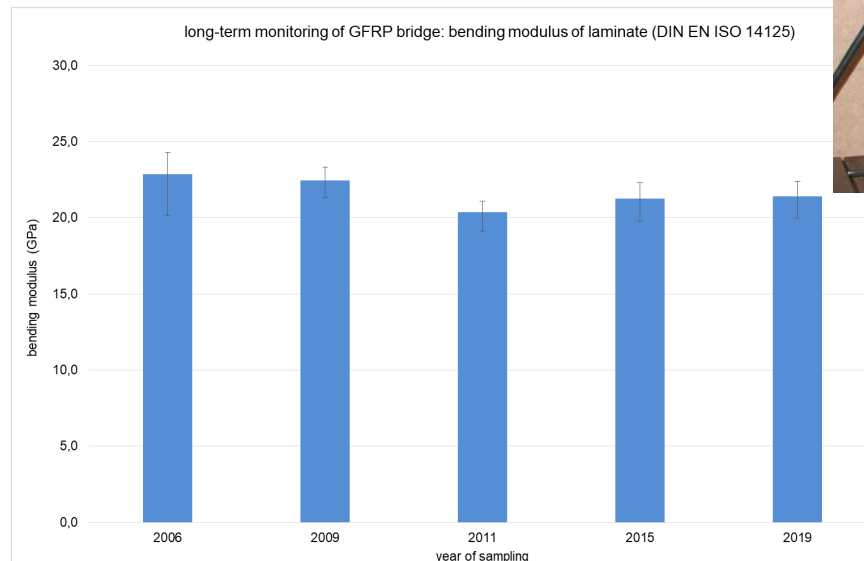
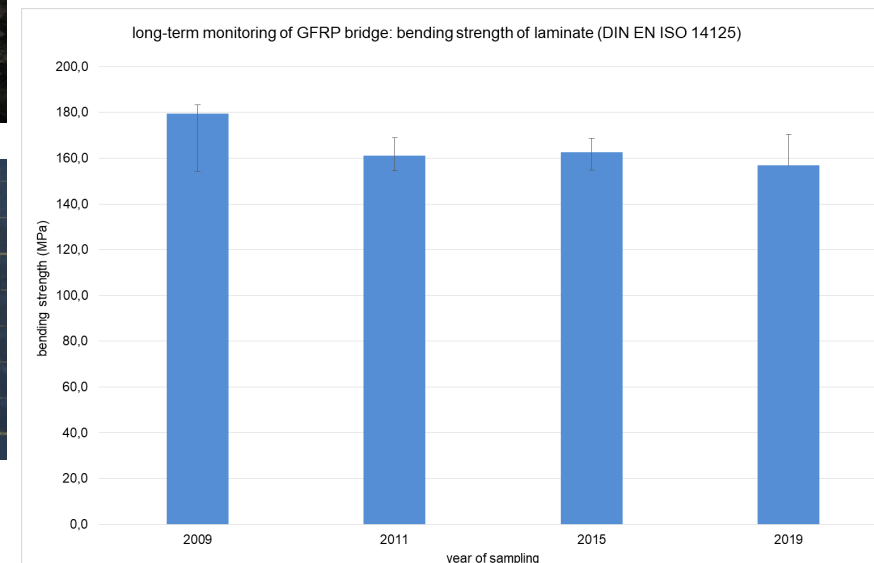
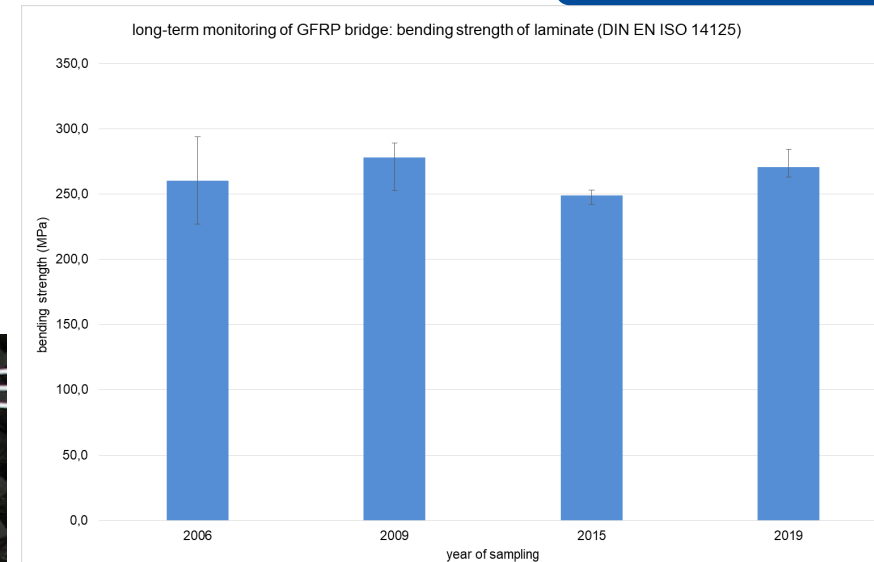
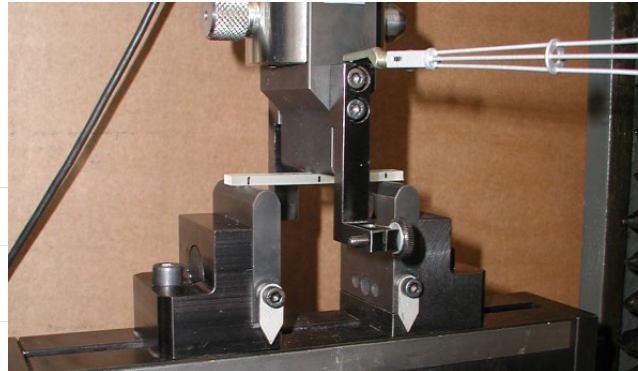


DLS-samples after the test

# Case Study 1: GFRP-Bridge

## Material properties for practice-aged samples:

- Samples taken from tested DLS-samples
- 3-point bending test following DIN EN ISO 14125
- Test until sample failure





# Case Study 1: GFRP-Bridge

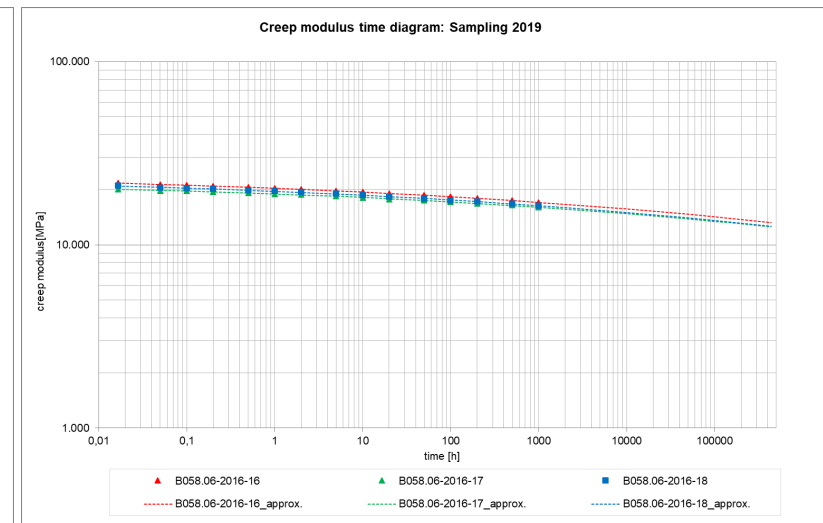
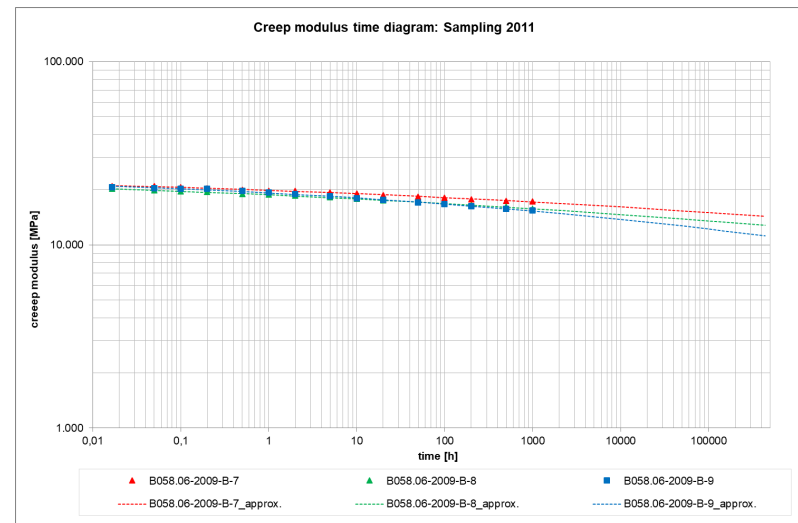
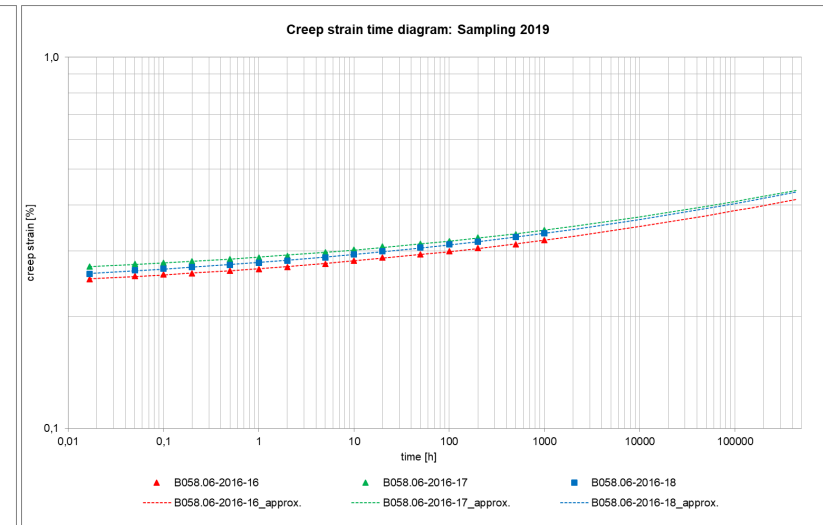
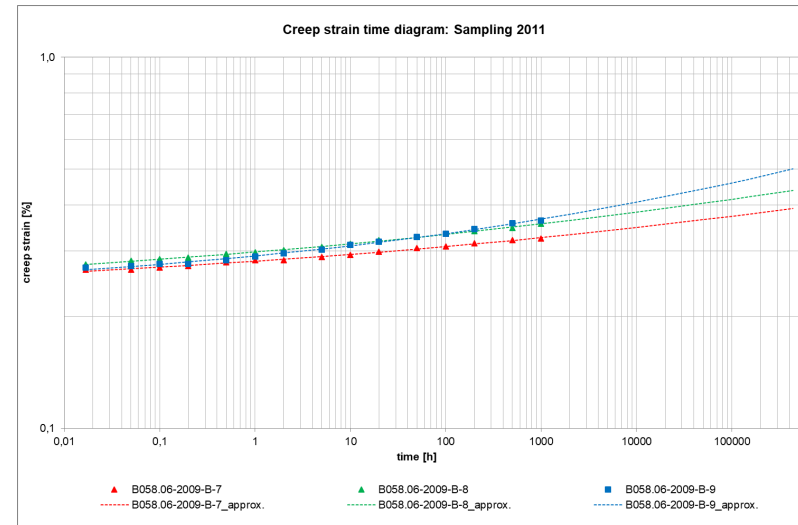
## Stiffness properties for practice-aged samples:

- Creep test
- 3-point bending
- Test duration: 1000 h
- Load level: 35% of static strength
- Approximation using: 3-parameter-exponentiation approach acc. Findley

y variable for elongation (mm)

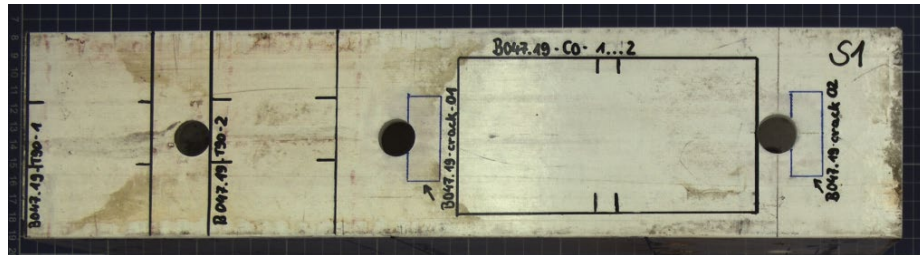
x variable for test time (h)

$k_1, k_2, k_3$  constants

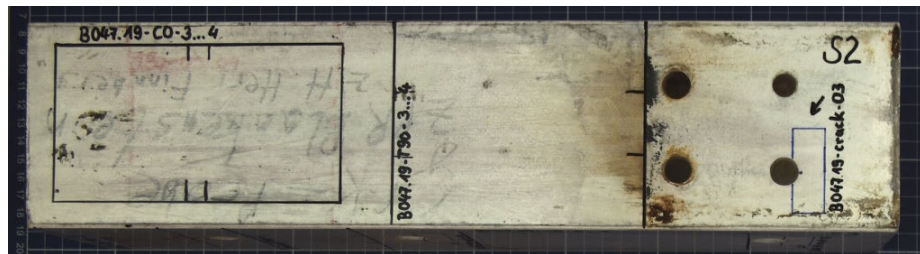


# Case Study 2: GFRP-Structured Cooling Tower

- 1<sup>st</sup> cooling tower with GFRP structure in Germany (Wet cooling tower with forced venting system)
- Built in 1999 with regular profiles of manufacturer Fiberline Composites A/S
- After approx. 20 years of service life, square-tube GFRP profile 100 x 100 x 8 mm were removed from the structure for testing



GFRP profile 100 x 100 x 8 mm with drill holes and markings for sampling



Structure of cooler tower with GFRP-profiles;  
Source: Fiberline A/S



Cooling tower of pulp and paper mill; Source: Fiberline A/S

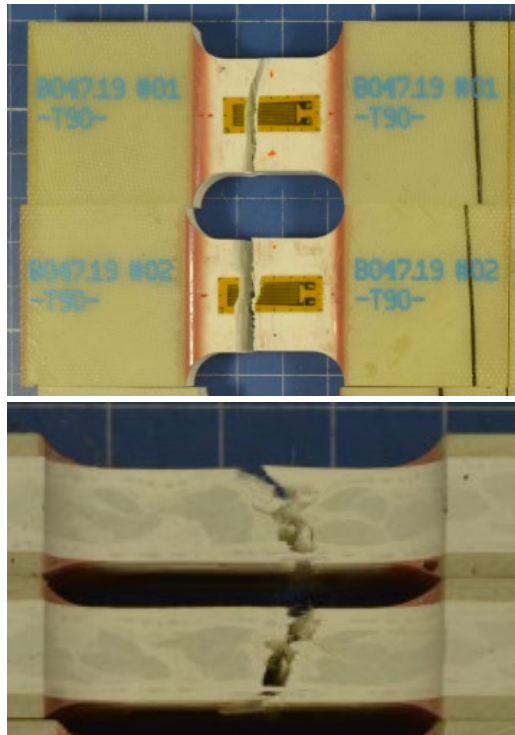


# Case Study 2: GFRP-Structured Cooling Tower



## Material Testing at Standard Climate of:

- Tensile test perpendicular to profile direction following DIN EN ISO 527-4



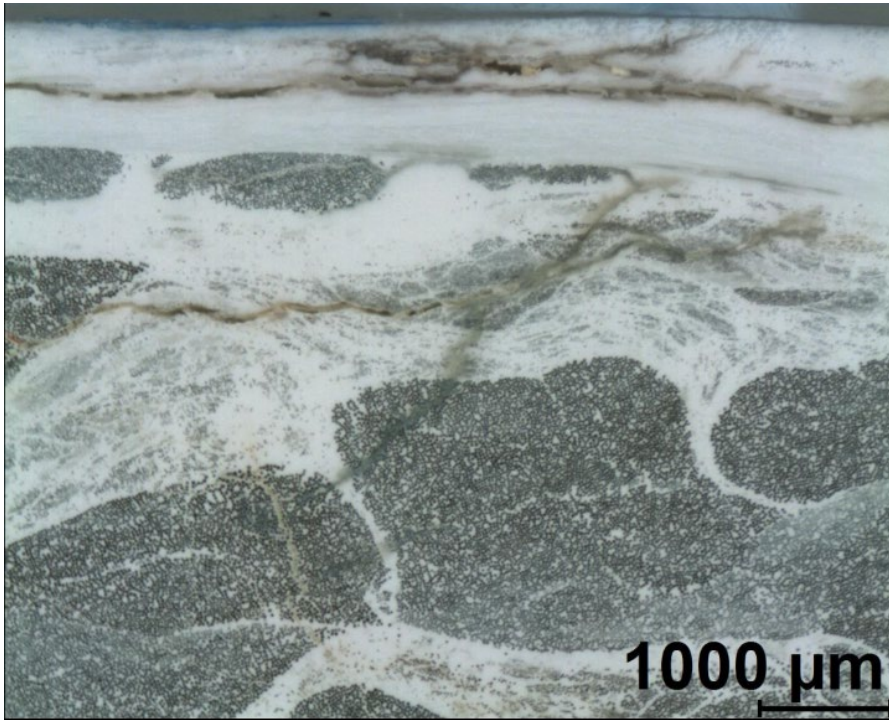
- Compression test parallel to profile direction





# Case Study 2: GFRP-Structured Cooling Tower

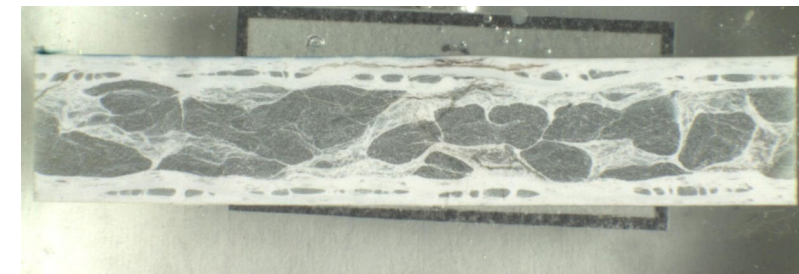
## Microscopy in the Area Surrounding the Drill Holes



Cutting depth approx. 1,4 mm



Cutting depth approx. 5,2 mm



- Crack depth is generally not large
- Negative influence of the cracks and delaminations on the load-bearing capacity of the profiles can be excluded
- Growth of the cracks to give a negative effect on load-bearing capacity is not expected

# Case Study 2: GFRP-Structured Cooling Tower

## Comparison of design values with properties after 24 years of use



- For dimensioning of structures made of GFRP creep and aging effects shall be taken into account at least via the following influencing factors:

- $A_1$ : Duration of exposure
- $A_2$ : Media influence
- $A_3$ : Ambient temperature

- For determination of the total reduction factor for strength and stiffness, the following approach was used, with the DIN 18820-2 valid during the construction of the cooling tower:

$$A_F = A_{F1} \times A_2 \times A_3 \quad \text{or} \quad A_E = \sqrt{A_1^E} \times A_2 \times A_3$$

- For construction profiles of company Fiberline A/S the following factors can be set for service life of 24 years:

Strength parallel to profile direction:  $A^F = A_{F1}^F \times A_2 \times A_3 = 1,3 \times 1,2 \times 1,1 = 1,72$

Stiffness parallel to profile direction:  $A^E = \sqrt{A_1^E} \times A_2 \times A_3 = 1,13 \times 1,2 \times 1,1 = 1,50$

Strength perpendicular to profile direction:  $A^F = A_{F1}^F \times A_2 \times A_3 = 1,9 \times 1,2 \times 1,1 = 2,50$

Stiffness perpendicular to profile direction:  $A^E = \sqrt{A_1^E} \times A_2 \times A_3 = 1,38 \times 1,2 \times 1,1 = 1,82$

- Determined influencing factors after a service life of 24 years are within the permissible properties of the profile

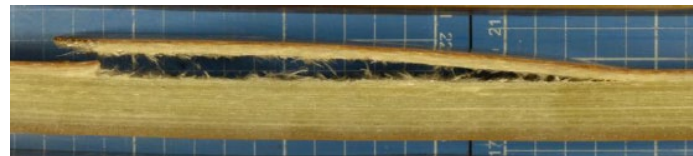


# Case Study 3: GFRP-Pipe at Power Plant

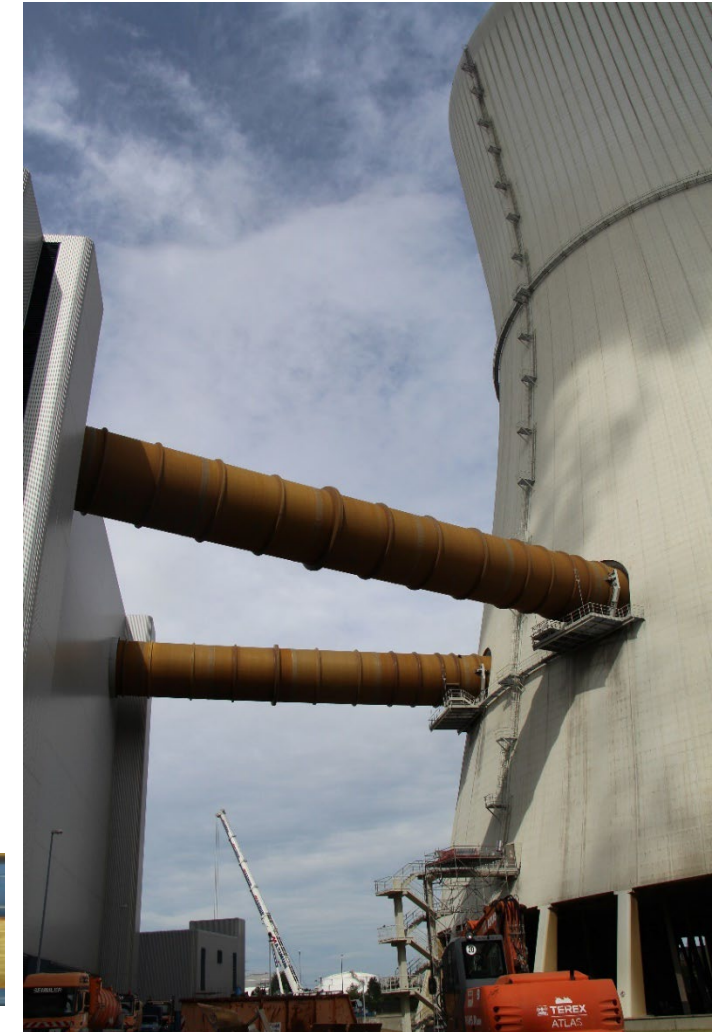
- Application clean gas channel (CGC) in coal power plant
- Use of GFRP since 1980
- GFRP have been used successfully applications with increased corrosive stress such as media pipes
- Issue: bubbles inside CGC
  - Root cause:  
formed in intermediate layers, when the substrate laminate is wrapped on a previously fully cured chemical protective layer
  - Mechanism:  
different coefficients of thermal expansion + thermal expansion + expansion of penetrated liquid
  - Additional effect:  
If insufficient adhesion between the layers, cavities are formed (bubbles on the surface)



Bubbles inside clean gas channel (CGC)



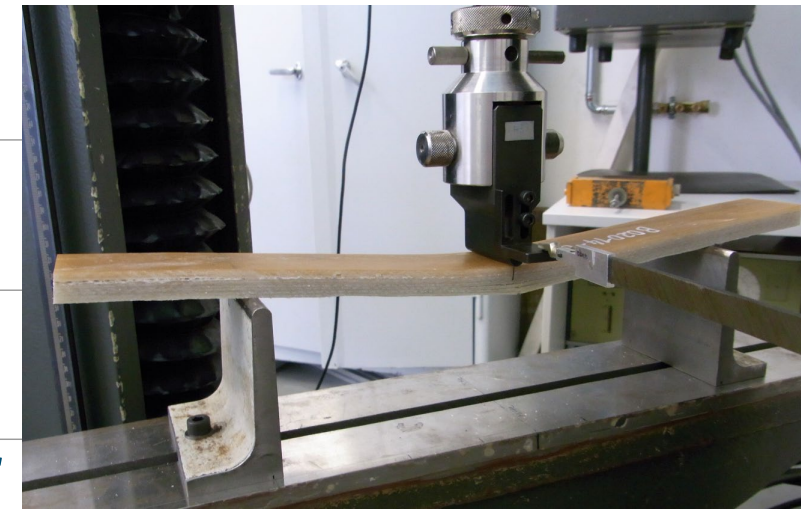
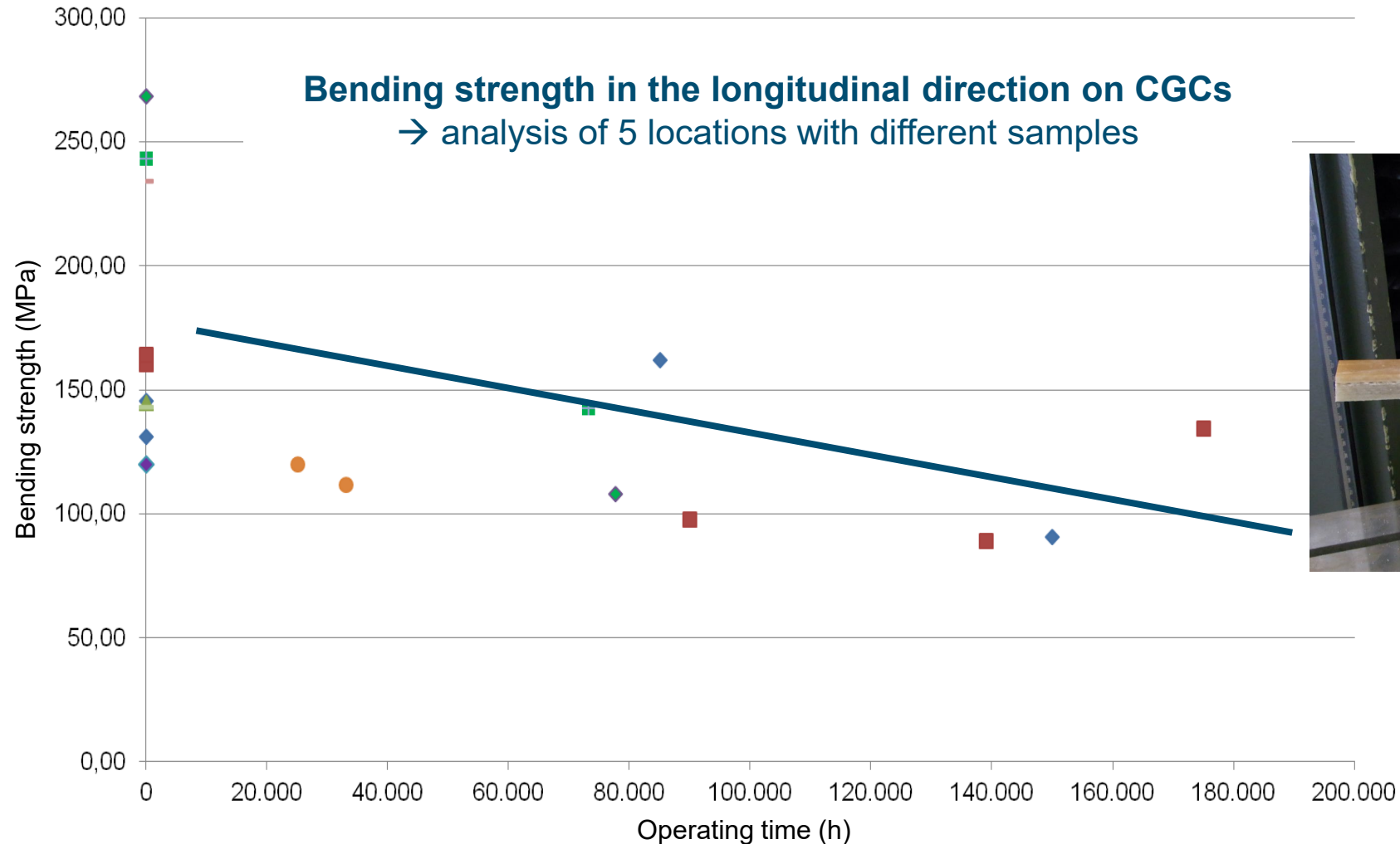
Cross section of bubble



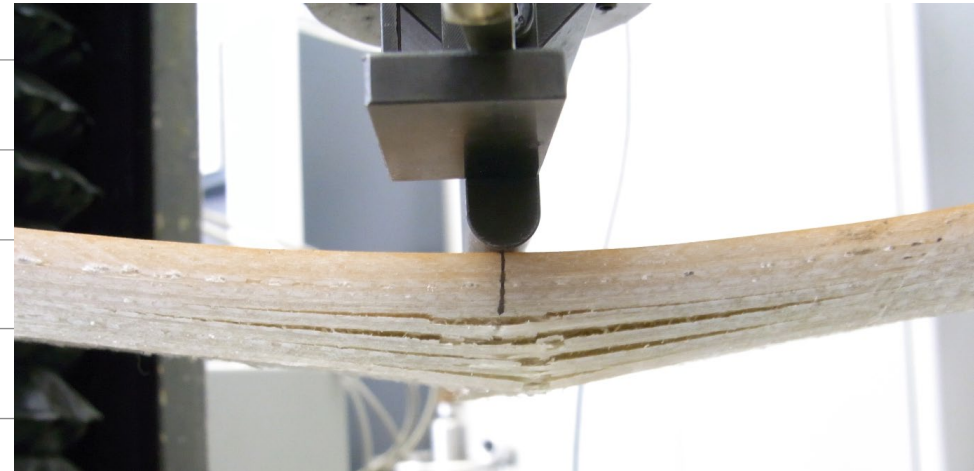
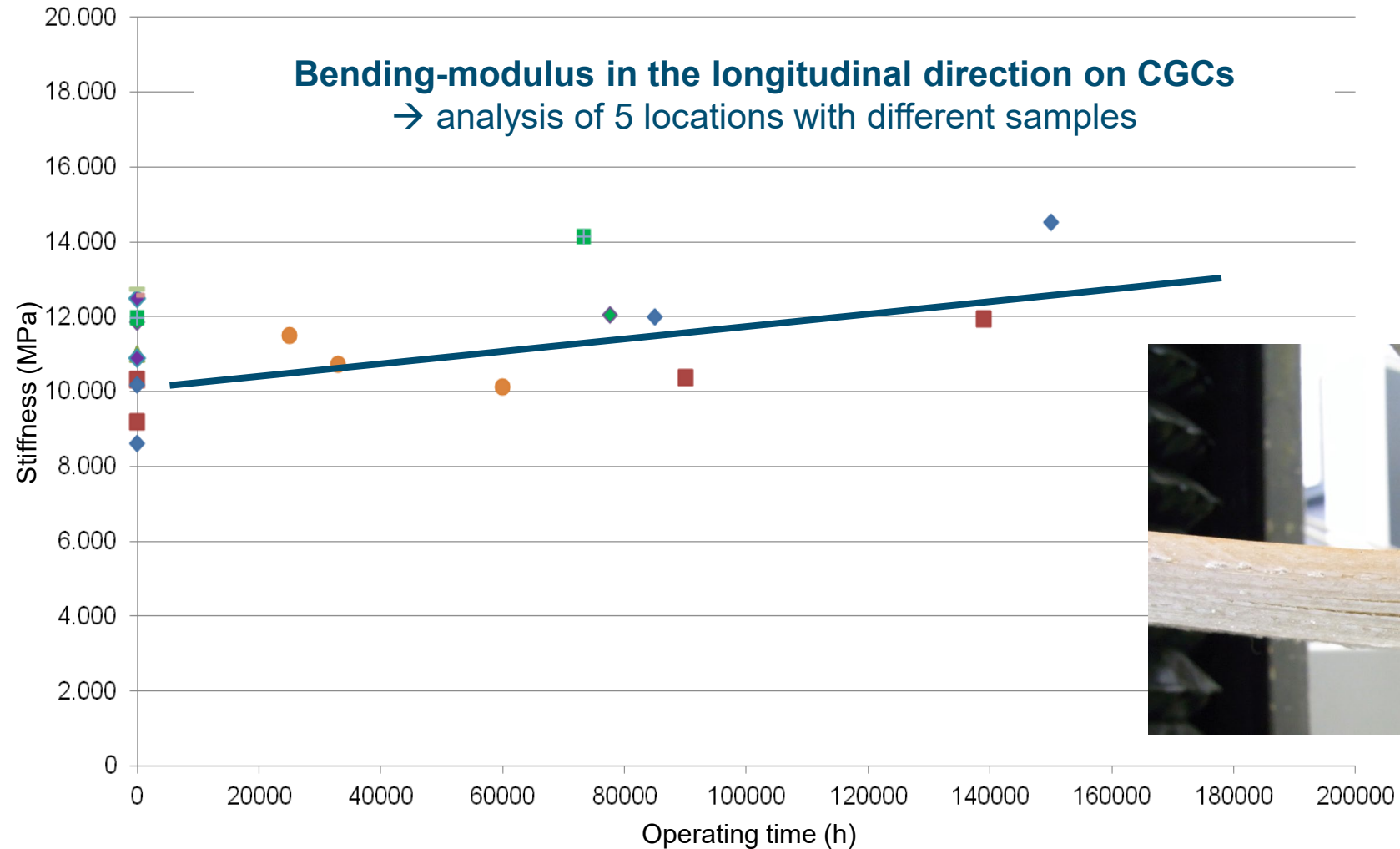
Clean gas channel (CGC) in brown coal power plant



# Case Study 3: GFRP-Pipe at Power Plant



# Case Study 3: GFRP-Pipe at Power Plant



# Case Study 3: GFRP-Pipe at Power Plant

## Comparison of design values with properties after 20 years of service life

- Following characteristics were set for the laminate with given lay-up structure:
  - Bending modulus (circumferential direction): 20.000 MPa
  - Bending modulus (longitudinal direction): 11.000 MPa
  - Bending strength (circumferential direction): 420 MPa
  - Bending strength (longitudinal direction): 145 MPa
  - Creep tendency (longitudinal direction): < 8%
- Factors of influence:
  - A1 (long-term load) = 1,4
  - A2 (media) = 1,33
  - A3 (temperature) = 1,35 → total reduction factor of  $A_1 \times A_2 \times A_3 = 2.51$  can be derived



→ Calculation of the reduction using factors of influence, CGC after 175,000 operating hours:

### circumferential direction

Property	After 175.000 h	Reduction $A_1 \cdot A_2$ according to 175.000 h	Reduction $A_3$	Reduction $A_1 \cdot A_2 \cdot A_3$ according to 175.000 h	Comparison
$E_F$ [MPa]	28.099	0,71	1,35	0,96	< 2,51
$\sigma_F$ [MPa]	276,4	1,52	1,35	2,05	< 2,51

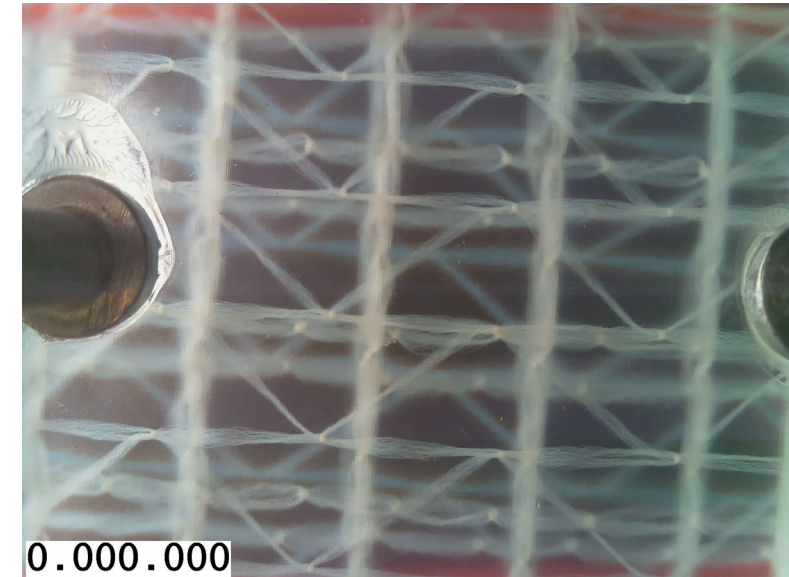
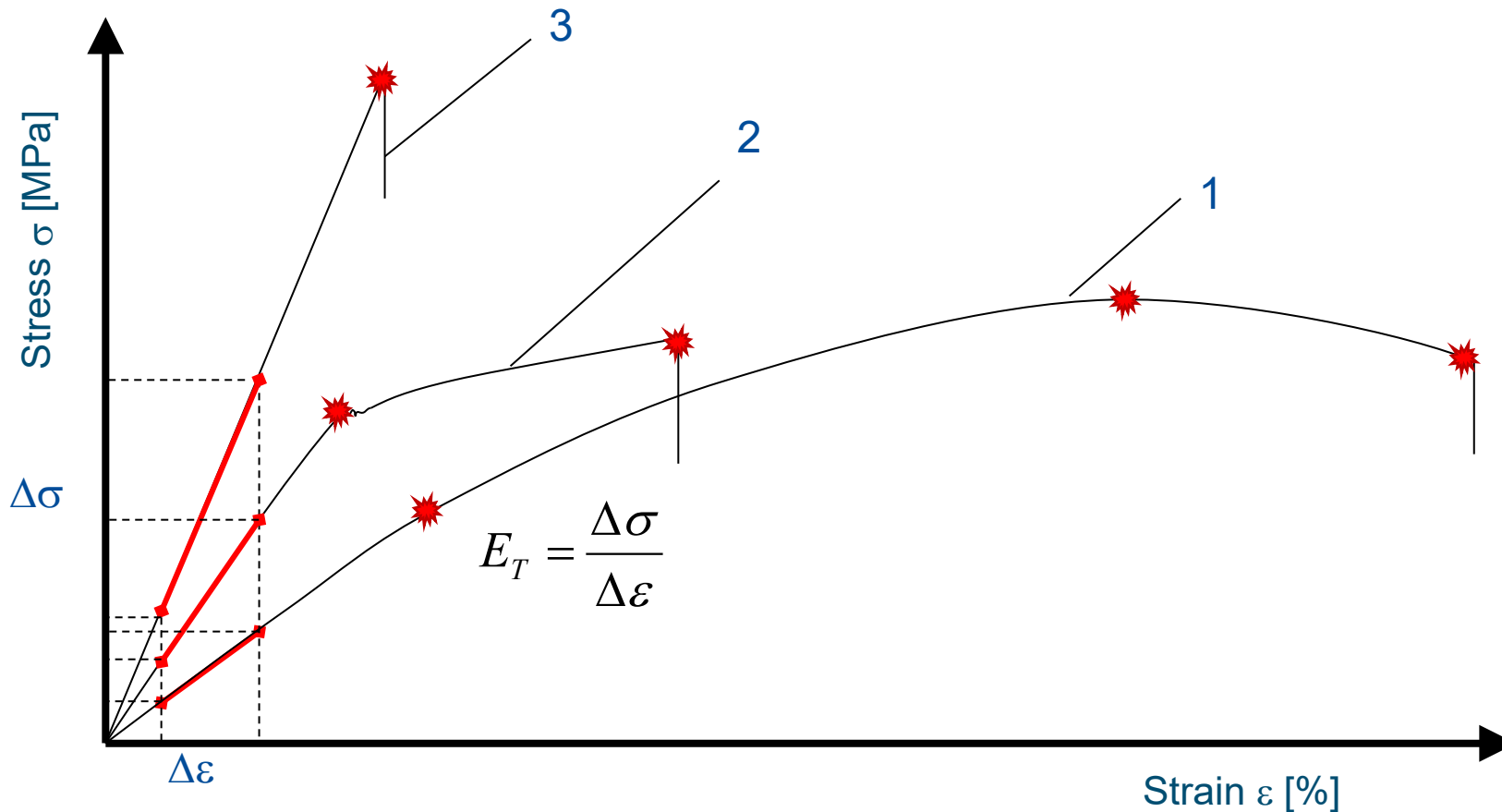
### longitudinal direction

Property	After 175.000 h	Reduction $A_1 \cdot A_2$ according to 175.000 h	Reduction $A_3$	Reduction $A_1 \cdot A_2 \cdot A_3$ according to 175.000 h	Comparison
$E_F$ [MPa]	17.887	0,61	1,35	0,82	< 2,51
$\sigma_F$ [MPa]	134,6	1,08	1,35	1,46	< 2,51



# Approach for Failure Determination during Fatigue Test

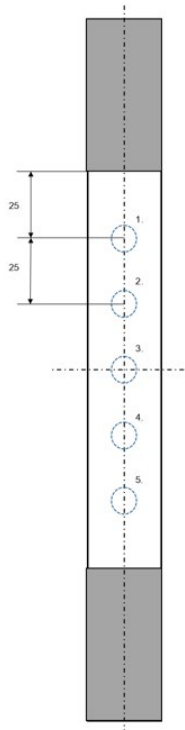
## General material behaviour of FRP



- 1...UD in 0°
- 2...Multiaxial (0°/90°)
- 3...BIAX

# Approach for Failure Determination during Aging Test

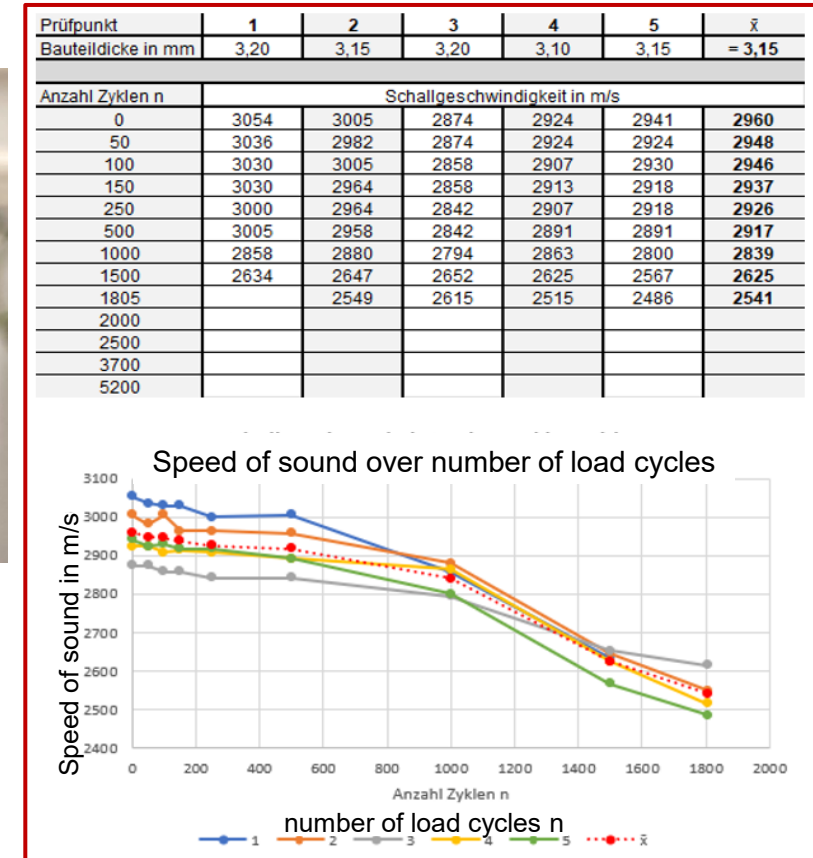
## Ultrasonic testing to estimate the fatigue behavior



Schematic layout of the test points and test setup



Coupon sample with NDT after 500 load cycles

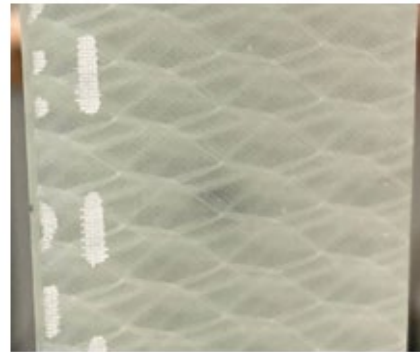
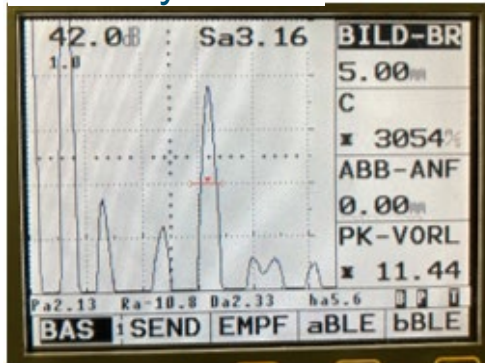


# Approach for Failure Determination during Aging Test

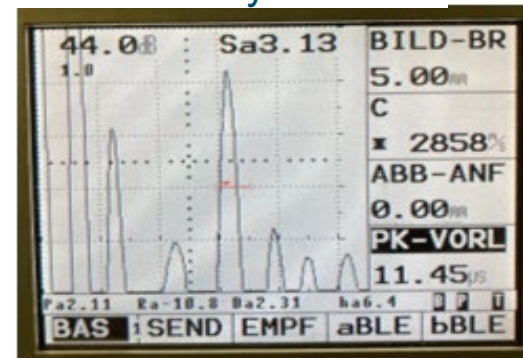


Change of ultrasonic testing signal due to increasing number of inter-fibre failures and growing delamination in the specimen

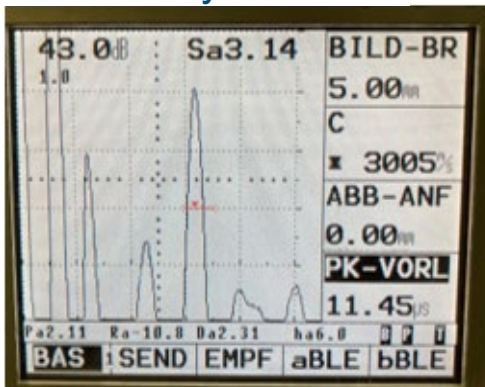
0 load cycles



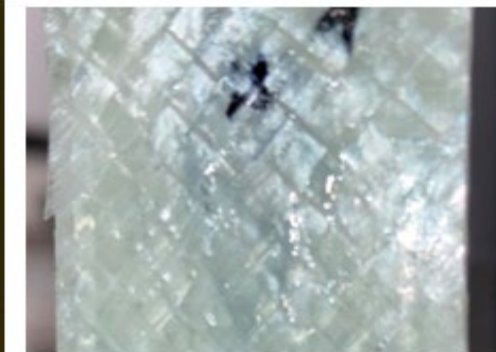
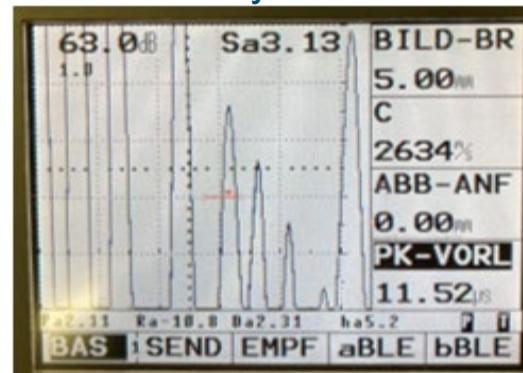
1.000 load cycles



500 load cycles



1.500 load cycles





# Approach for Failure Determination during Aging Test



Aim:

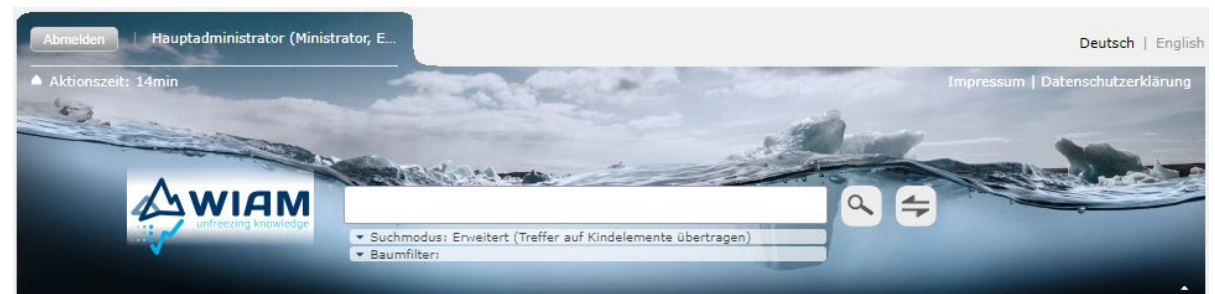
- Evaluation of the material condition and derivation of predictions on the remaining service life
- Increasing operational safety through recurring monitoring or permanent monitoring in critical areas

Suitable NDT methods for structural monitoring of GFRP components:

- Ultrasonics (UT)
- Guided waves (AU)
- Acoustic emission (AE)

Requirement:

- Numerous test data for different material
- Consideration of load types / load direction
- Data basis for the evaluation of NDT-images



# Summary



1. With an optimised choice of reinforcement fibre, layer structure, matrix and, if necessary, fillers to meet the requirements of the application, construction projects with the highest demands can be realised over a long period of time (>25 years).
2. Applications of GRP from all industries and technologies allow us to expect service lives of 50 - 100 years.
3. The actual degradation of the material depends not only on the loads and the choice of material, but also on the manufacturing quality (degree of cross-linking) and on the assembly (residual stress).
4. I.e. the degradation factors required for the design are not purely theoretically determinable factors! They can differ greatly in practice, because they depend on the manufacturing quality.
5. The use of "witness samples" help to determine the long-term behaviour more reliably.
6. Non-destructive testing methods are suitable for structural monitoring and evaluation of the material condition.

**With our materials and pultruded profiles, we are well positioned in the competition of materials.**

## Contact:

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